



## **Mechanical Properties of High volume Fly ash concrete**

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**Abstract :** An experimental investigation was carried to study the mechanical properties of the high volume fly ash concrete. Fly ash is a by-product of burning pulverized coal. M<sub>40</sub> mix was prepared with various Fly ash replacement propositions of cement such as 0%, 40%, 50%, 55 % and 60% with 0.330 water cement ratio. These test results were noted at 7, 28, 56 days intervals. Based on the results the 50 % replacement of fly ash with cement gave higher compressive strength, hence the result is followed for split tensile strength and flexural strength[9]. From the results, enhancement of Mechanical properties of high volume fly ash concrete attained at the later stage as compare to the Reference concrete.

**Keywords :** High volume fly ash (HVFA), Ordinary portland cement(OPC), Split tensile, Flexural.

### **1.0 Introduction**

In developing country like India depends on the production value of power and consequently its consumption as energy, Fossil fuel plays an important part in meeting the demand for power generation .Coal is considered to be one of the world's richest and widely distributed fossil fuel. Fly ash is a by-product of burning pulverized coal in an electrical generating station. It is a finely-divided amorphous alumino-silicate with varying amounts of calcium, which when mixed with portland cement and water, will react with the calcium hydroxide released by the hydration of portland cement to produce various calcium-silicate hydrates (C-S-H) and calcium-aluminate hydrates [1]. Some fly ashes with higher amounts of calcium will also display cementitious behavior by reacting with water to produce hydrates in the absence of a source of calcium hydroxide. Both of these mechanisms enhance the durability of the concrete [12]. From the study mechanical properties of the high volume fly ash replaced cement concrete was compared to the reference concrete[8].

### **2.0 Experimental investigation:**

**2.1 Cement** - Ordinary Portland cement of 53-grade in Ultra tech brand was used. Opccement was tested in accordance with the IS: 4031-1968[5].

**2.2 Fine Aggregate** – River sand is collected from locally available area. Physical properties of the fine aggregate were tested with respect to IS 2386-1963. Particle size distribution of sand shows, it is in Zone II of IS 383-1970 [4].

**2.3 Coarse Aggregate** –Maximum size of 20 mm crushed blue granite stone is collected from the locally available area. Physical properties of the Coarse aggregate were tested with respect to IS 2386-196.[5]

**2.4 Fly ash** – Class F Fly ash was imported from Eklahare thermal Power Station from Nashik. It is branded as Dirk Pozzocrete 60. Which confines as per IS 3812 (Part 2) – 2003 [6].

**2.5 Superplasticizer** –Polycarboxylate ether based superplasticizer is used to reduce the water content as well improve the workability of the concrete. This is named as Master Glenium sky 8233.

**2.6 Water** – Potable water.

**Table 1 M<sub>40</sub>Mix Proposition**

Mix	Cement (kg)	Fly Ash (%)	Fly Ash (kg/m <sup>3</sup> )	Fine Aggregate (kg/m <sup>3</sup> )	Coarse Aggregate (kg/m <sup>3</sup> )	w/c	water kg/m <sup>3</sup>	super plasticizer (0.5 %)
1	448	0	0	732	1125	0.330	148	2.240
2	268	40	180	732	1125	0.330	148	2.240
3	224	50	224	732	1125	0.330	148	2.240
4	246	55	202	732	1125	0.330	148	2.240
5	180	60	268	732	1125	0.330	148	2.240

### 3.0 Mix Design

Mix design is carried out for a particular compressive strength of concrete with adequate workability so that the fresh concrete can be properly mixed, placed and compacted. As per IS 10262: 2000 [2] mix of M<sub>40</sub>(1: 1.678: 2.538) was preferred for the replacement of cement with high volume of fly ash [7]. Mix propositions are shown in table 1.

### 4.0 Casting and Curing of specimens

The Cubes were Casted at 150x150x150 mm for Compressive Strength, Cylinder of 150 mm diameter, 300 mm length for tensile strength and 100x100x500 mm of prism for flexural strength and demoulded after 24 hours, immersed in a curing tank after that test were done at 7, 28, 56 days interval.

## 5.0 Results and discussion

### 5.1 Slump cone test

Slump cone test is used determine the Workability property of High volume fly ash concrete. Fly ash is added to improve the workability of the concrete and similar workability values were observed by T.sankaralingam [10].A spherical shape fly ash particle helps to reduce friction between aggregates and concrete [3]. From the table 2, slump value was increased gradually with respect to the fly ash concentration.M<sub>4</sub> mix have a high workability as compare to other mixes.

**Table 2 Slump value**

Mix	M <sub>1</sub>	M <sub>2</sub>	M <sub>3</sub>	M <sub>4</sub>	M <sub>5</sub>
Slump Value(mm)	102	118	138	160	178

### 5.2 Compressive Strength

Compressive strength for the mix M<sub>40</sub> with the various fly ash replacement of cement, results given in the table 3 and Fig 1.Compressive strength was calculated at 7, 28, 56 days. In a long time period fly ash based concrete attains an optimum result. At 7 days results the reference concrete (M<sub>0</sub>) gains the higher strength but from 7 to 56 days of time intervals M<sub>2</sub> mix gains higher strength as compare to the other replacements[10]. Though 56 days compressive strength for M<sub>2</sub> replacement is higher than the reference concrete. M<sub>2</sub> mix values are higher than M<sub>0</sub> by 5.01%.Fig 2 shows the partial replacement (M<sub>2</sub>) of fly ash with cement gains the higher compressive strength, Similiar strength was obtained by G.Venkatesan and S.Ragu [11]. From the Fig 1 represents the graphical model for the test results. From the model observed M<sub>0</sub>, M<sub>1</sub>, M<sub>2</sub>, M<sub>3</sub> mix gives above 50 Mpa compressive strength. The compressive strength was calculated by using the following formula.

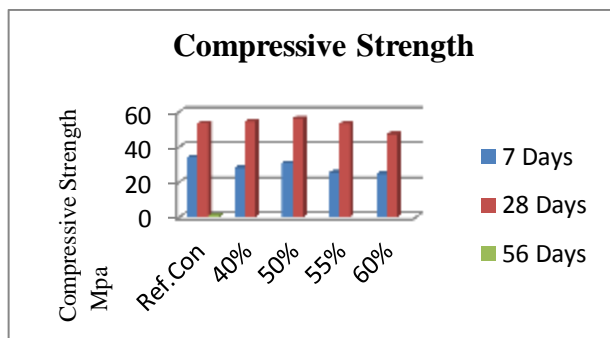
$$\text{Compressive Strength} = \frac{\text{Load}}{\text{Area}}$$

**Table 1 Compressive strength**

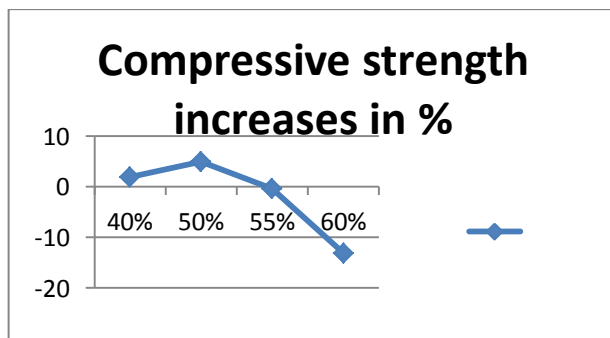
Mix	Fly Ash Replace ment %	Compressive Strength In Mpa			Com. strength Increase in %
		7 days	28 days	56 Days	
M0	0	34.22	50.32	53.4	
M1	40	28.10	47.01	54.45	1.95
M2	50	30.80	48.61	56.08	5.01
M3	55	25.50	46.52	53.20	-0.37
M4	60	24.62	41.22	47.20	-13.14

### 5.3 Split Tensile Strength

Split Tensile Strength was calculated at 28, and 56 days of Reference concrete and various replacement of fly ash with cement, results given in the table 4 and Fig 3,4. From the graph and table, partially replaced fly ash (M<sub>2</sub>) based concrete has attained optimum strength at 28, 56 days of time periods [13]. But in 56 days of tensile strength of M<sub>3</sub> and M<sub>4</sub> mix decreased as compare to the reference concrete.



**Fig 1 Compressive strength**



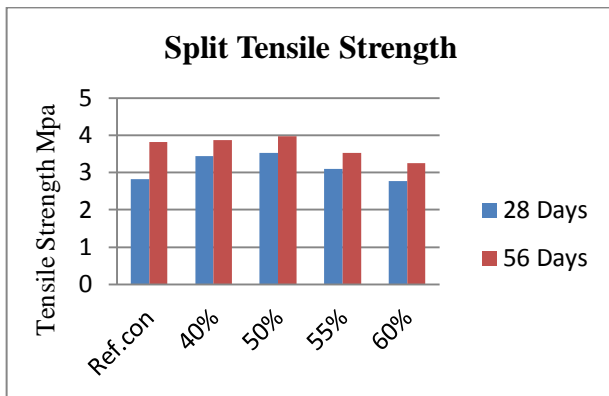
**Fig 2 Compressive strength increases**

The tensile strength has increased 3.93 % in M<sub>2</sub> mix as compared to the M<sub>0</sub> at 56 days of time period. Increment of tensile strength was begins at M<sub>1</sub> and M<sub>2</sub> mixes at 28 and 56 days. The split tensile strength was calculated by using the following formula.

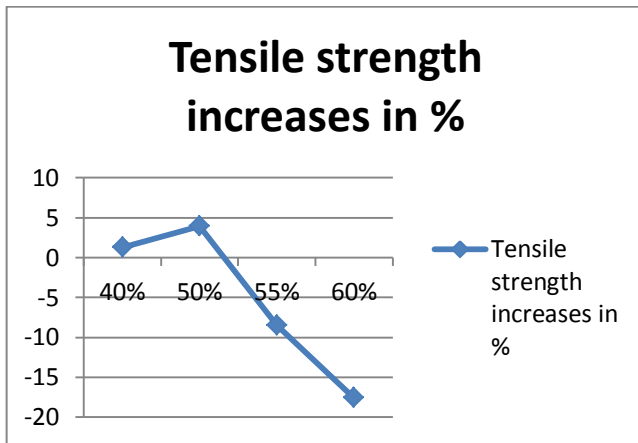
$$f_{st} = 2P / \pi ld$$

**Table 4** Split Tensile Strength

Mix	Fly Ash Replacement %	Split Tensile Strength		Split Tensile Strength Increases %
		28 days	56 Days	
M0	0	2.83	3.82	
M1	40	3.45	3.87	1.30
M2	50	3.52	3.97	3.93
M3	55	3.10	3.52	-8.52
M4	60	2.78	3.25	-17.53



**Fig 3** Split Tensile Strength



**Fig 4** Split Tensile Strength

**5.4 Flexural Strength**

Flexural Strength of prism was calculated at 28 days of Reference concrete and various replacement of fly ash with cement, results given in the table 5 and Fig 5. From the table flexural strength was obtained only in M<sub>1</sub> and M<sub>2</sub> mixes as compared to M<sub>0</sub>, but M<sub>3</sub> and M<sub>4</sub> results fallen suddenly as compare to the M<sub>2</sub> mix. M<sub>4</sub> mix result fallen 39 % from the M<sub>0</sub> value. The Flexural strength was calculated by using the following formula.

$$\text{Flexural strength} = \frac{P \times L}{B \times D^2}$$

P - ultimate load in KN

## 6.0 Conclusion

1. It is concluded that sufficient work ability was attained for M<sub>40</sub> grade of concrete at 0.330 water cement ratio and 0.5 % of super plasticizer content.
2. From the Results, higher compressive strength was attained at 50% replacement of fly ash (M<sub>2</sub>) with cement at 56 days of curing and M<sub>3</sub>mix has an similar strength as compared to the reference concrete.
3. It results shows, high volume fly ash concrete attained higher strength at a later days of curing. The similar trends were followed in split tensile and flexural strength.

## 7.0 Reference

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