



International Journal of ChemTech Research CODEN (USA): IJCRGG ISSN : 0974-4290 Vol.6, No.2, pp 938-947, April-June 2014

Fully Replacement of Natural Sand With hydro Sluiced Bottom Ash in High Performance Concrete with Nanosilica

A Jayaraman¹*

¹Department of Civil Engineering Bannari Amman Institute of Technology, Sathamangalam-638401, India.

> *Corres. Author: jayaraman.cvl@gmail.com Cell : +91 7502105223, Phone : +91 4295 226000 (Ext: 6113) Fax : +91 4295 226666

Abstract: Generally the river sand is used as a fine aggregate in concrete and is obtained by mining the sand from river bed. Increased sand mining not only affects the aquifer of the river bed but also causes environmental problems. In recent days the demand for river sand is increasing due to its lesser availability. Hence the practice of replacing river sand with hydro sluiced bottom ash is taking a tremendous growth. It is also inferred from the literature that replacement of normal sand with hydro sluiced bottom ash produces no appreciable increase in compressive strength due to the variation in the pore size of concrete at micro level. This paper presents the optimization of fully replacement of hydro sluiced bottom ash by natural sand with nano silica in high performance concrete. The ordinary Portland cement is partially replaced with nano-silica by 0.25%, 0.5 %, 0.75% and natural sand is fully replaced with hydro sluiced bottom ash. The studies reveal that the increase in percentage of partial replacement of nano silica increased the compressive, tensile and flexural strength of concrete. It was found that 0.55 water/cement ratio produced higher compressive strengths, tensile strength and better workability for partially replaced with nano-silica by 0.75 % mix, proportion. Specifically compressive and tensile strength ranged from 21.06 -35.2 N/mm² and 10.06 -15.5 N/mm² for the mixes considered. These results compare favourably with those of conventional concrete.

Keywords: compressive strength, flexural strength, bottom ash, nano silica, tensile strength.

I INTRODUCTION

Concrete is the second most consumed substance on Earth after water. Sand only production is growing by 8.2 % annually, and is expected to rise from 8.44 billion usages to 12.4 - 13.56 tonnes by 2050. The demand of natural sand is quite high in the developing countries due to the rapid infrastructural growth. Normally particles are not present in river sand up to required quantity. Digging sand, from river bed in excess quantity is hazardous to environment. The deep pits dug in the river bed, affects the ground water level. In order to fulfill the requirement of fine aggregate, some alternative material must be found. The cheapest and the easiest way of getting substitute for natural sand is obtained from bottom ass, limestone quarries, lateritic sand and crushing natural stone quarries is known as manufactured sand. The bottom ash is a waste material that is generated from power plants (NLC). Concrete made using partial replacement of bottom ash 20% and manufacturing sand of 25%, 50%, 75%, 100% for cement and fine aggregate respectively. The compressive strength of concrete increases when percentage of manufacturing sand increases up to 75% and decreases when 100% replacement. The split tensile strength varies marginally.¹ The various strength properties studied consist of compressive strength, flexural strength and splitting tensile strength. The strength development for various percentages (0-50%) replacement of fine aggregates with bottom ash can easily be equated to the strength development of normal concrete at various ages.² The mechanical properties of special concrete with 30% WBA replacement by weight of natural sand is found to be an optimum usage in concrete in order to get a favourable strength and

good strength development pattern over the increment ages.³ In NLC, About 1.3 million tons of fly ash is being generated every year as a waste product in all three power plants. The disposal of fly ash is posing a problem to the thermal power station. Normally fly ash is disposed as slurry in ash ponds. This kind of disposal makes the land as a waste land and cannot be used any purposes. However utilizing the fly ash potential as a pozzolanic material, fly ash is being extensively used by the NLC in construction activities apart from being supplied to cement industries, brick manufactures at free of cost. Coal bottom ash (CBA) and fly ash were utilized in partial replacement for fine aggregates and cement respectively in the range of 0, 5, 10, 15 & 20% (equal percentages). The results of compressive strength at 7, 28, 56 & 90 days curing are presented because of the pozzolanic reaction; other properties investigated include physical properties, fresh concrete properties and density. The results showed that for a grade 35 concrete with a combination of CBA and fly ash can produce 28 day strength above 30 MPa.⁴. The impacts of other nano materials such as CNTs, nano TiO₂, nano Al₂O₃ and nano TiO on concrete are also promising. While nano materials acts as fillers and provide nucleation sites for cement hydration, nano SiO₂ also acts as a pozzolanic material, increasing the amount of stiff CSH within the hydrated cement paste, resulting an improved microstructure .The concrete produced with 10% micro- SiO₂ and 2% nano- SiO_2 show higher degree of quality in their compressive strength, less water absorption and more electrical resistance in comparison with the others.⁵ The ordinary Portland cement is partially replaced with silica fume and natural sand is replaced with manufactured sand by four proportions. The results indicated that there is an increase in the compressive and flexural strength.⁶ Limestones are generally obtained from the calcareous remains of marine or fresh water organisms embedded in calcareous mud. They change from the soft chalks to hard crystalline rocks. The use of limestone as a concrete aggregate has sometimes been suspect on account of the unsuitability of the poorer grade rocks, and also because of a widespread fallacy that limestone concrete is less resistant to the action of fire than concrete made from other aggregates. Laterite stone is commonly used for the construction purpose. There are several laterite stone quarries in Konkan region. During excavation of laterite stone, around 25–30 per cent laterite stone scrap is generated. It is estimated that about 2.83 cum of the laterite stone scrap is generated during excavation of about 11.33 cum of the laterite stone. This laterite stone scrap creates problem in quarries and needs removal for further excavation. In order to add value to this waste material, it is felt necessary to manufacture the blocks using different constituents that are suitable for the construction. In this In this overview determine the Compressive strength, toughness index and water absorption capacity of the laterite stone scrap blocks.⁷ The natural sand was replaced with coal bottom ash by 0%, 10%, 20%, 30%, 40%, 50%, 60%, 70%, 80%, 90%, and 100% by weight, as water absorption of bottom ash was more so that quantity of water was increased to achieve 100 mm slump. The results shows that the compressive strength, split tensile strength and flexural strength decreased as the percentage of replacement coal bottom ash increased as compared to controlled concrete.⁸ By practice it shows that conventional mix has more strength than bottom ash sand mix so we will be adding various % of nanosilica to the bottom ash mix and compare the values of compressive strength, corrosion resistance, tensile strength and economy in practice between the conventional and the M sand with 0.75% nano silica mix. Since we are replacing nanosilica with cement the strength increases and the porosity decreases as the nano materials fill the fine pores between the aggregates and the cement. The practicality for its usage in the marine environment, Nuclear power plants, Chemical Industry, etc are studied.

II EXPERIMENTAL INVESTIGATION

2. Materials Used

2.1. Cement: Ordinary Portland cement (OPC) 43 grade conforming to IS 8112 – 1989, and specific gravity of cement is found to be 3.15. The properties of cement given in Table 1.

2.2. Fine aggregates: Local clean river sand fineness modulus of medium sand equal to 2.36 conforming to grading zone of III of IS: 383-1970. The specific gravity of fine aggregate is 2.54. The sieve analysis of fine aggregate given in Table 2.

2.3. Hydro Sluiced Bottom Ash: Hydro Sluiced Bottom ash is replaced is fully replacement of river sand .It is collected from NLC India. The bulk density of bottom ash 1360 kg/m² and the specific gravity 2.47 and fineness modulus of bottom ash is 2.36.the maximum size of the aggregate is 4.75 mm. The testing of bottom ash is done as per Indian standard specifications IS: 383 - 1970. The properties of Bottom Ash given in Table 3.

2.4. Course aggregates: Locally available well graded aggregates of normal size greater than 4.75 mm and less than 16 mm having a fineness modulus of 2.72 was used as coarse aggregates. The aggregates are tested as per the procedure given in BIS: 2386

2.5. Nano-silica: The nano silica is partially replaced for cement. Steel Authority of India has provided necessary facilities to produce annually about 3000 tons of nano silica at their Bhadravathi Complex. In India, However, the nano silica of International quality is marketed by Elkem Metallury (P) Ltd. The properties of nano silica given in Table 4.

| Physical properties of cement | | | |
|--------------------------------|-----------|--|--|
| Initial setting time(minutes) | 55 mins | | |
| Final setting time(minutes) | 275 mins | | |
| Standard consistency | 31.0% | | |
| Specific gravity | 3.14 | | |
| Fineness of cement | 10% | | |
| Chemical properties of cement | | | |
| SiO ₂ | 20 - 21 | | |
| Al ₂ O ₃ | 5.2 - 5.6 | | |
| Fe ₂ O ₃ | 4.4 - 4.8 | | |
| CaO | 62 - 63 | | |
| MgO | 0.5 - 0.7 | | |
| SO ₃ | 2.4 - 2.8 | | |
| Loss on ignition (LOI) | 1.5 - 2.5 | | |

Table 1: Properties of OPC Cement

Table - 2: Sieve analysis of fine aggregate

| IS | sieve | Weight of retained | River sand % | Requirement |
|-------------|-------|--------------------|--------------|---------------|
| designation | | on sieve (gm) | Passing | IS:383 - 1970 |
| 10mm | | 0 | 100 | 100 |
| 4.75 mm | | 16 | 98.4 | 90 - 100 |
| 2.36mm | | 82 | 90.2 | 85 - 100 |
| 1.18mm | | 150 | 75.2 | 75-100 |
| 600nm | | 133 | 61.9 | 60-100 |
| 300um | | 298 | 32.1 | 13 - 40 |
| 150um | | 257 | 6.2 | 0 - 10 |

| Chemical ingredient | % by mass |
|--------------------------------|-----------|
| SiO_2 | 79.65 |
| Al ₂ O ₃ | 14.71 |
| Fe ₂ O ₃ | 3.20 |
| CaO | 0.39 |
| MgO | 0.55 |
| SO ₃ | 0.70 |
| Loss on ignition (LOI) | 0.67 |

| Physical properties of Nano - Silica | | |
|--------------------------------------|--|--|
| Specific Surface Area | 200 m^2 / g to 300 m^2 / g | |
| Particle Size | ≤ 1 | |
| Specific gravity | 0.13 to 0.14 | |
| Chemical Composition of | of Nano – Silica | |
| Si O ₂ | 94.3 | |
| $Al_2 O_3$ | 0.58 | |
| $Fe_2 O_3$ | 0.30 | |
| Ca O | 0.24 | |
| Mg O | 0.44 | |
| K ₂ O | 0.67 | |
| Na ₂ O | 0.36 | |

Table 4: Properties of Nano - Silica

III EXPERIMENTAL PROCEDURE

The mix ratio is prepared for M30 grade concrete for both conventional sand and also Bottom ash Sand. The Cube size of (150 x 150 x 150) mm Specimen is prepared for compressive strength. The cylinder of height 30 cm and 15 cm diameter is prepared for tensile strength. The specimens are tested for 7 days, 14 days and 28 days with each proportion of nano silica and bottom ash Sand mix. Totally there are 45 cubes and 45 cylinders are casted. The specimen size of (70x10x10) cm is used for flexural strength test. For durability test mortar specimen is prepared in a mix ratio of 1:3, the cube size of (50 x50 x 50) mm is prepared for water absorption test, acid penetration test and for RCPT a concrete disc of size 85 mm diameters and 50 mm thickness. The specimen is tested 28 days totally for 30 cubs and 15 cylinders. All the specimens are demoulded after 24 hours, and curing is done in water for 7 days, 14 days and 28 days. The Scanning Electron Microscope (SEM) is used to determine the particle size of Nano –silica. Figure1.A Shows the SEM picture of NC Concrete, Figure1.B SEM picture of Bottom ash Sand Concrete and Figure1.C SEM picture of Bottom ash Sand with 0.75% of Nano-silica.



IV RESULT AND DISCUSSION

4.1. Compressive strength of concrete

The test is carried out conforming to IS 516 -1959 to obtain compressive strength of concrete at the 7days, 14 days and 28 days. The cubes are tested using 400 tonne capacity HELICO compressive testing machine (CTM) .The results are presented in Fig.2.



Fig.2. Compressive strength of concrete

The 7days compressive strength of bottom ash concrete 59 .2 % of compressive strength is reduced when compared to the normal river sand and of bottom ash with 0.75% nano silica concrete. The compressive strength of normal river sand and bottom ash with 0.75% nano silica concrete is more or less same and bottom ash with 0.5% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 46.9% of compressive strength is reduced when compared to the normal river sand and of bottom ash with 0.75% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete found to be 38.9 % and bottom ash with 0.25% nano silica concrete.

The 14 days compressive strength bottom ash concrete found to be 27%, bottom ash with 0.25% nano silica concrete found to be 30% found and bottom ash with 0.5% nano silica concrete to be 21.91% of compressive strength is reduced when compared to the bottom ash with 0.75% nano silica concrete and normal river sand concrete. The compressive strength normal river sand and bottom ash with 0.75% nano silica concrete is more or less same.

The 28 days compressive strength of bottom ash with 0.75% nano silica concrete are found to be 4.71% more than the normal river sand concrete and fully replacement of bottom ash concrete, bottom ash with 0.25% nano silica concrete and bottom ash with 0.5% nano silica concrete found to be 37.76%, 33.31% and 38.48% of compressive strength is reduced when compared to the bottom ash with 0.75% nano silica concrete.

Results of this test are show in table .5.

| S. | Type of mix | 7Days | 14 Days | 28 Days |
|----|-------------------------------------|----------|----------|-------------------|
| No | | Strength | Strength | Strength |
| | | N/mm^2 | N/mm^2 | N/mm ² |
| 1 | Conventional mix | 33.13 | 35.25 | 40.12 |
| 2 | Fully Bottom ash Sand Mix | 13.51 | 26.16 | 26.22 |
| 3 | Fully Bottom ash +0.25% Nano-silica | 17.56 | 25.15 | 28.15 |
| 4 | Fully Bottom ash +0.5% Nano silica | 20.23 | 28.25 | 30.13 |
| 5 | Fully Bottom ash +0.75% Nano silica | 32.16 | 36.18 | 42.13 |

Table 5: Compressive strength of concrete

4.2. Tensile strength of concrete

The test is carried out conforming to IS 516 -1959 to obtain tensile strength of concrete at the 7 days, 14 days and 28 days. The cylinders are tested using 400 tonne capacity HELICO compressive testing machine (CTM). The results are presented in Fig.3.



Fig.3.Tensile strength of concrete

The 7days tensile strength of bottom ash concrete found to be 27 .5 % and bottom ash with 0.25% nano silica concrete found to be 26.64 % of tensile strength is reduced when compared to the bottom ash with 0.75% nano silica concrete and tensile strength of normal river sand and bottom ash with 0.75% nano silica concrete and bottom ash with 0.5% nano silica concrete is more or less same.

The 14 days tensile strength of bottom ash concrete found to be 24 .11 % and bottom ash with 0.25% nano silica concrete found to be 21.86 % of tensile strength is reduced when compared to the bottom ash with 0.75% nano silica concrete and tensile strength of normal river sand and bottom ash with 0.75% nano silica concrete is more or less same.

The 28 days tensile strength of bottom ash with 0.75% nano silica concrete are found to be 6.31% more than the normal river sand concrete and fully replacement of bottom ash concrete, bottom ash with 0.25% nano silica concrete and bottom ash with 0.5% nano silica concrete found to be 13.26%, 12.08% and 4.8% of tensile strength is reduced when compared to the bottom ash with 0.75% nano silica concrete. Results of this test are show in table .6.

| S. | Type of mix | 7Days | 14 Days | 28 Days |
|----|-------------------------------------|----------|----------|-------------------|
| No | | Strength | Strength | Strength |
| | | N/mm^2 | N/mm^2 | N/mm ² |
| 1 | Conventional mix | 2.52 | 2.82 | 3.1 |
| 2 | Fully Bottom ash Sand Mix | 1.89 | 2.36 | 2.87 |
| 3 | Fully Bottom ash +0.25% Nano-silica | 1.92 | 2.43 | 2.91 |
| 4 | Fully Bottom ash +0.5% Nano silica | 2.59 | 3.06 | 3.15 |
| 5 | Fully Bottom ash +0.75% Nano silica | 2.61 | 3.11 | 3.31 |

Table 6: Tensile strength of concrete

4.3. Flexural strength of concrete

The test is carried out conforming to IS 516 -1959 to obtain flexural strength of concrete at the 7days, 14 days and 28 days are tested using loading frame 750 kN. The results are presented in Fig.4.



Fig.4.Flexural strength of concrete

The 7days flexural strength of bottom ash concrete found to be 16.48 % and bottom ash with 0.25% nano silica concrete found to be 14.40 % of flexural strength is reduced when compared to the bottom ash with 0.75% nano silica concrete and flexural strength of normal river sand and bottom ash with 0.75% nano silica concrete and bottom ash with 0.5% nano silica concrete is more or less same.

The 14 days flexural strength of bottom ash concrete found to be 14 .85 % and bottom ash with 0.25% nano silica concrete found to be 14.97 % of flexural strength is reduced when compared to the bottom ash with 0.75% nano silica concrete and flexural strength of normal river sand and bottom ash with 0.75% nano silica concrete is more or less same.

The 28 days flexural strength of bottom ash with 0.75% nano silica concrete are found to be 7.9% more than the normal river sand concrete and fully replacement of bottom ash concrete, bottom ash with 0.25% nano silica concrete and bottom ash with 0.5% nano silica concrete found to be 17.29%, 26.98% and 8.69% of flexural strength is reduced when compared to the bottom ash with 0.75% nano silica concrete. Results of this test are show in table .7.

| S. | Type of mix | 7Days | 14 Days | 28 Days |
|----|-------------------------------------|----------|----------|-------------------|
| No | | Strength | Strength | Strength |
| | | N/mm^2 | N/mm^2 | N/mm ² |
| 1 | Conventional mix | 7.21 | 8.19 | 10.27 |
| 2 | Fully Bottom ash Sand Mix | 6.13 | 7.11 | 9.23 |
| 3 | Fully Bottom ash +0.25% Nano-silica | 6.28 | 7.10 | 8.15 |
| 4 | Fully Bottom ash +0.5% Nano silica | 7.12 | 8.21 | 10.19 |
| 5 | Fully Bottom ash +0.75% Nano silica | 7.34 | 8.35 | 11.16 |

Table 7: Flexural strength of concrete

4.4. Water absorption test

This test is done as per procedure given in ASTM C 642-97 by oven-drying method. The results are presented in Fig.5. For this test 50mm x 50mm x 50mm cubes are cast. After 24 hours of remolding, the specimens are kept immersed in water. At the end of 28 days, the specimens are taken from the curing tank and air-dried to remove the surface moisture then taken the initial weight (W1) is taken. The final weight (W2) is taken to the specimens are dried in an oven at a temperature of $100+10^{\circ}$ C for 48 hrs, and allowed to cool at room temperature.

The 28 days water absorption of bottom ash with 0.75% nano silica concrete are found to be 38 %, 41%, 34.08%, and 13.80% of water absorption is reduced when compared to the normal river sand, fully replacement of bottom ash, bottom ash with 0.25% nano silica concrete and bottom ash with 0.5% nano silica concrete. Bottom ash with 0.75% nano silica concrete is best impermeability of concrete compare to other mix ratio. Nano silica is the good durability of concrete. Results of this test are show in table .8.

% of Water absorption

% of water absorption = $[(W2 - W1)/W1] \times 100$ Where,

W1 = weight of oven dried sample in air.

W2 = weight of surface dry sample in air after immersion in water.



Fig.5. % of water absorption test

| Table 8: | % | Water | absorption | test |
|----------|---|-------|------------|------|
|----------|---|-------|------------|------|

| S.No | Type of mix | % Water absorption test |
|------|-------------------------------------|-------------------------|
| 1 | Conventional mix | 6.65 |
| 2 | Fully Bottom ash Sand Mix | 6.99 |
| 3 | Fully Bottom ash +0.25% Nano-silica | 6.25 |
| 4 | Fully Bottom ash +0.5% Nano silica | 4.78 |
| 5 | Fully Bottom ash +0.75% Nano silica | 4.12 |

4.5. Acid penetration test

This test is done as per procedure given in ASTM C 642-97 by oven-drying method. The results are presented in Fig.6. For this test 50mm x 50mm x 50mm cubes are cast. After 24 hours of remolding, the specimens are taken the initial weight (W1) after kept immersed in HCL (pickling solution). At the end of 28 days, the specimens are taken the finial weight (W2) is taken. The 28 days acid penetration of bottom ash with 0.75% nano silica concrete is high resistance in permeability and high durability of concrete compare to the other mix ratio. Results of this test are show in table .9.

4.6. Rapid chloride penetration test

This test is conducted as per ASTM C1202-09. The results are presented in Fig.7. Concrete disc of size 85 mm diameters and 50 mm thickness average value of three samples. The specimens are carried at different stages, allowed to cure for 28 days and then they are subjected to RCPT test by impressing a voltage of 60v. The permeability of fully replacement of bottom ash mix is very high when compared to the bottom ash with 0.75% nano silica concrete and conventional mix. Bottom ash with 0.75% nano silica concrete is to found 35.34% ,49.29% ,36.32% and 46.95 % found to be to less than normal river sand, fully replacement of bottom ash, bottom ash with 0.25% nano silica concrete and bottom ash with 0.5% nano silica concrete. There is a significant improvement in the durability of concrete because of high pozzolanic nature of the nano-silica and its void filling ability. The RCPT values show in table 10.

The columbs value ≥ 4000 the Chloride Ion high. In between 2000 – 4000 the Chloride Ion moderate, 1000 – 2000 the Chloride Ion low, In between 100 – 1000 the Chloride Ion very low and ≤ 100 negligible.

The constituents are weighted and the material is mixed by hand mixing. The mixes are compacted using table vibration. The water binder ratio (W/B) adopted is 0.375 concrete and mortar. The specimens are demoulded after 24h, cured in water for 7, 14 and 28 days and then tested for its compressive, tensile and durability test as per indian standards. There is a significant improvement in the strength of concrete because of high pozzolanic nature of the nano silica and its void filling ability.



Fig.6. Acid penetration test

Table 9: Acid penetration test

| S.No | Type of mix | % Acid penetration test |
|------|------------------------------------|-------------------------|
| 1 | Conventional mix | 10.34 |
| 2 | Fully Bottom ash Sand Mix | 14.34 |
| 3 | Fully Bottom ash +0.25% Nano- | 10 |
| | silica | |
| 4 | Fully Bottom ash +0.5% Nano silica | 4.78 |
| 5 | Fully Bottom ash +0.75% Nano | 4.12 |
| | silica | |



Fig.7.Rapid chloride penetration test

Table 10: Rapid chloride penetration test

| S.No | Type of mix | % Chloride Ion passing |
|------|-------------------------------------|------------------------|
| 1 | Conventional mix | 652.5 |
| 2 | Fully Bottom ash Sand Mix | 831.86 |
| 3 | Fully Bottom ash +0.25% Nano-silica | 795.28 |
| 4 | Fully Bottom ash +0.5% Nano silica | 662.5 |
| 5 | Fully Bottom ash +0.75% Nano silica | 421.86 |

V. CONCLUSION

Addition of nano-Silica leads to a significance increase in the characteristic strength and durability of concrete.

Replacement of cement with 0.75% of nanosilica gives more strength than the bottom ash mix and also the durability has been increased compared to the bottom ash sand Mix. The self weight of the Nano mix is lighter than the bottom ash sand and the conventional mix. The workability decreases with the addition of Nano-Silica compared to the conventional mix and the bottom ash sand mix. The penetration level of chlorides and acids are less in Nano concrete compared to that of conventional and fully replacement of bottom ash sand.

REFERENCES

- 1. C.Mathiraja (2013) "Mechanical Properties of concrete using bottom ash and M.Sand". International Journal of Latest Trends in Engineering and Technology (IJLTET),2013.
- 2. P. Aggarwal, Y. Aggarwal, S.M. Gupta (2007) "Effect of bottom ash as replacement of fine Aggregates in concrete." Asian journal of civil engineering (building and housing) vol. 8, no. 1 (2007) Pages 49-62.
- 3. Mohd Syahrul Hisyam bin Mohd Sani, Fadhluhartini bt Muftah, Zulkifli Muda (2010)"The Properties of Special Concrete Using Washed Bottom Ash (WBA) as Partial Sand Replacement". International Journal of Sustainable Construction Engineering & Technology Vol 1, No 2, December 2010.
- 4. Abdulhameed Umar Abubakar, Khairul Salleh Baharudin (2012) "Properties of concrete using tanjung bin power Plant coal bottom ash and fly ash" International Journal of Sustainable Construction Engineering & Technology (ISSN: 2180-3242) Vol 3, Issue 2, 2012.
- 5. Mohammad Reza Zamani Abyaneh, Seyed Mahdi Mousavi, Alireza Mehran, Seyed Mohammad, Mahdi Hoseini, Soroosh Naderi, Farhad Mesbah Irandoost,(2013). "Effects of Nano silica on Permeability of concrete and steel Bars Reinforcement Corrosion." Australian Journal of Basic and Applied Science,7(2):464-467, 2013.
- 6. T.Shanmugapriya, R.N.Uma,(2012)."Optimation of Partial Replacement of M-Sand by Natural sand in High Performance Concrete With Silica Fume." International Jouranal of Engineering Sciences & Emerging Technologies, June 2012.Volume 2, pp:73.-80 IJESET.
- 7. S.K. Jai1, P.G. Patil, N.J. Thakor,(2011) "Engineering properties of laterite stone scrap blocks". Agricultural Engineering International: CIGR Journal. Vol.13, No.3, 2011. Manuscrip No. 1738.
- M.P.kadam, Dr.Y.D. Patil (2013) ". Effect of coal bottom ash as sand replacement of the properties of concrete with different w/c ratio." International Journal of Advanced Technology in Civil Engineering, ISSN: 2231 –5721, Volume-2, Issue-1, 2013.
