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# Comparative Study on Durability, Mechanical Strength and Ecology of Ferrocement Made from Geopolymer and Conventional Portland Cement Mortar

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**Abstract :** Ferrocement is a composite material composed of a mortar reinforced with steel fabric/mesh, used to form thin sections. Geopolymer is an innovative construction material prepared by utilizing the industrial waste materials like Fly ash, GGBS, Red mud, Rice husk ash etc... In the present study OPC with partial replacement of Fly Ash (25%, 50%) and geopolymer mortar were taken with the geopolymer mortar ratio of 1:2. And the liquid/binder ratio was 0.45. It was observed that ferrocement made by geopolymer mortar shown superior properties interms of strength,durability,fatigue,hightemperatureresistance,corrosion,etc. Corrosion was not observed even after 60 days of Alternate Wetting and Drying (AWD) cycles of Accelerated Corrosion Test. The test results show that flexural strength of geopolymer mortar increases with increase in volume fraction percentage and specific surface of steel mesh as compare to ferrocement mortar. The micro-structural changes were determined by FESEM. These new composites (Geopolymer mortar) can be used form a king precast elements such as casting of water tanks, innovative architectural aesthetic constructions, faster low cost durable houses, corrosion resistant pipes for sewage lines, tunnel linings in metrorail, roads and irrigation techniques. The present development of new ferrocement can become an apt substitute for conventional ferrocement, but with very low carbon footprint.

**Keywords:** Ferrocement, Geopolymer, wire mesh, Accelerated corrosion test, Durability, FESEM, EE, ECO2E.

### Introduction:

Ordinary Portland cement(OPC) is one of the most essential binder material in the construction industry. It is estimated that the production of OPC, releases the large amount of Carbon-di-oxide (CO<sub>2</sub>) gas which subsidizes the 65% of global warming [1,2]. For the production of one ton of OPC emits approximately 0.8 ton of CO<sub>2</sub> in the atmosphere, thereby the cement industry produces 7% of all CO<sub>2</sub> emission. Although the use of OPC is unavoidable in the construction industry, many efforts were made in order to find an alternative/to reduce the consumption of OPC.The research work is going in the utilization of accompanying cementious materials such as fly ash, GGBS, Red mud, rice husk ash, metakaolin as a binder in the construction world.In this aspects Davidovits coined a new innovative technology namely “Geopolymer” which shows a significant promise for application in the concrete industry, in terms of reducing the global warming through CO<sub>2</sub> emission and also alternative to the OPC [3].Geopolymer mortar is an inorganic polymer prepared from the industrial waste by-products(Contains alumino-silicate) like fly ash and it is activated by the alkaline solutions[4].Class F fly ash based geopolymer matrix possess high mechanical strength, moderately low creep , possess excellent thermal resistant and also resistant to the acid, sulphate attack[5,6].

The two main constituents in the preparation of GP ferrocement are source material and alkaline liquid. The source material for GP ferrocement should be rich in aluminium(Al) and silicon(Si) and also availability, cost of the precursor material is very essential[8]. The most commonly used alkaline solution for the geopolymerisation is a hybrid combination of sodium hydroxide and sodium silicate solutions. Forming a shape with the required structure in the construction field a ferrocement is required. Ferrocement(FC) is an environmental friendly in nature and it also had a superior properties in terms of cracks control, impact resistance and toughness largely due to the closely spacing and uniform dispersion of reinforcement within the material [9] because of its light weight structural thin member, which is suitable for substituting wood and for structural elements of exotic shapes such as shells, chimneys, boats, etc [10-15].FC is an gorgeous construction method for its unique properties like tensile strength, fire resistant ,earthquake and corrosion compared to the traditional method. The main benefits of utilizing this FC in construction field are due to its light weight, minimum maintenance and long lifetime.

In the present study the ferrocement are made with conventional cement mortar with 25% and 50 % replacement of fly ash and geopolymer mortar. In the present study three types of conventional ferrocement and ferrogeopolymer mortar matrix were formulated .The matrix durability was studied by ACT method and ecological importance were studied in this research paper.

## 2. Materials:

### 2.1 Cement:

Generally, the ferrocement is prepared by using OPC.As per the Indian Standard IS 12269-1987 of grade 53 cement was used in the present study. The Physical Properties of solid materials has been done and it is tabulated in **table1**.

**Table1 Physical Properties of Solid Source Materials**

Material	Property	Result
OPC (Tests as per IS:4031)	Conforming BIS Code	IS:12269
	Fineness Index(90 $\mu$ m sieve)	4.0%
	Fineness Index(Blaine)	342 m <sup>2</sup> /Kg
	Specific Gravity	3.13
	Normal Consistency	31%
	Initial setting time	45 minutes
	Compressive strength(IS:4031)	55 MPa
Fly Ash	Final setting time	225 minutes
	Conforming BIS Code	IS:3812
	Type	Class F
	Fineness(Blaine)	342 m <sup>2</sup> /Kg
	Specific Gravity	2.21
GGBFS	Bulk density	1010 kg/m <sup>3</sup>
	Conforming BIS Code	IS:3812
	Fineness(Blaine)	425 m <sup>2</sup> /Kg
	Specific Gravity	2.91
Fine aggregate -Riversand	Bulk density	1100 kg/m <sup>3</sup>
	Conforming BIS Code	IS:383
	Fineness Modulus	2.5
	Water Absorption	0.8%
	Specific Gravity	2.51
	Bulk density	1610 kg/m <sup>3</sup>

## 2.2 Ground granulated blast furnace slag (GGBS):

The GGBS used in the study was obtained from JSW Cements Ltd. The Specific surface area of the slag was 425m<sup>2</sup>/Kg. GGBS is an industrial waste product from Steel manufacturing Industry.

## 2.3 Fly ash:

In the present study class F fly ash obtained from Ennore power plant, Tamilnadu is used. The Specific surface area of the fly ash was 342m<sup>2</sup>/Kg. The chemical composition of powdery material was listed in **table2**.

**Table2 Chemical Composition of Solid Source Materials**

Component	SiO <sub>2</sub>	Al <sub>2</sub> O <sub>3</sub>	Fe <sub>2</sub> O <sub>3</sub>	Na <sub>2</sub> O	CaO	MgO	K <sub>2</sub> O
Fly ash	47.5	33.4	10.1	0.01	2.09	0.1	1.6
GGBS	21.6	14.9	1.78	0.01	55.3	2.6	0.5
Cement	21.1	4.5	0.55	0.6	65.02	2.9	0.4

## 2.4 Fine Aggregate:

Fine aggregate is used to produce FC. Normal river sand with the fineness modulus of 2.45 was used as fine aggregate and it is obtained from nearby river beds. Based on the fine aggregate selection the consistency and compaction is achieved in the FC matrix.

## 2.5 Water:

Water is used for the hydration of the binder material(OPC). The pH of the water should be maintained as 7 and it should be free from organic and deleterious matter will fit for the construction purpose.

## 2.6 Activating solution(AS):

The combination of Na<sub>2</sub>SiO<sub>3</sub>(MR SiO<sub>2</sub>/Na<sub>2</sub>O: 2.0, wherein Na<sub>2</sub>O 15%, SiO<sub>2</sub>33% and H<sub>2</sub>O 52%) and caustic lye (48% NaOH) were used in the study. The activating solution is prepared at least 24 hrs. prior to use. The physio-chemical properties of activating solution are listed in table3.

**Table.3 Physio-chemical properties of activating solution**

Chemical formula	Na <sub>2</sub> O:SiO <sub>2</sub>
Appearance	Liquid(Gel)
Color	Colorless liquid
Boiling point	102 <sup>0</sup> C for 40% aqueous solution
Specific gravity	1.58
Molecular weight	184-250
Density	1.54 kg/l

## 2.7 Steel wire mesh:

This Consist of one layer of steel wire mesh of 25 mm x 25mm gauge length and 3 mm thick, with tensile strength of 512N/mm<sup>2</sup> covered on either side by a layer of chicken wire mesh of 0.5mm thickness with opening area 150 mm<sup>2</sup> has been used as reinforcement

## 3. Experimental method and test procedure:

### 3.1 Compression and Split tensile strength:

Plastic Cylindrical moulds of size:50mm diameter x 100mm height was used to cast the specimens. Demoulding of these cylindrical moulds was done by cutting carefully the plastic with thick sharp steel cutting

tool and the specimens were submerged in water and air curing. Three mix proportions were used in the preparation of the ferrocement. Cement to sand ratio was fixed to 1:2 and the water to cement ratio was maintained as 0.45 for all the mixes. The cementitious material (OPC) was replaced by 25% and 50% of fly ash. Mixing and curing were performed according to the ASTM C192-02. Eventually Three geopolymer mixes have been studied Geopolymeric source material (GSM: GGBS and Fly ash) to sand ratio was fixed to 1:2 and the Activating solution (AS) to binder was 0.45 for structural grades. The GGBS/ (FA+GGBS) 1, 0.75, and 0.5 are taken as GSM in the ferropolymer mortar mixes. The GSM and fine aggregate were mixed together for 3-4 mins followed by the AAS in the mixer pan and mixing was continued for 5mins. The mix design is tabulated in table4

**Table.4 Mix design of ferrocement mortar.**

Mix Id	Type		CM1	CM <sub>2</sub>	CM <sub>3</sub>	GM <sub>1</sub>	GM <sub>2</sub>	GM <sub>3</sub>
Binding Materials	Cement	%	100	75	50	-	-	-
	FA	%	-	25	50	-	25	50
	GGBS	%	-	-	-	100	75	50
AAS	Type	A Mixture of SHF and SSS						
	SiO <sub>2</sub> /Na <sub>2</sub> O	w/w	-	-	-	0.56	0.56	0.56
L/B Ratio	w/w	w/w	0.45	0.45	0.45	0.45	0.45	0.45
Materials Quantity kg/m <sup>3</sup>	Cement	-	640	480	320	-	-	-
	FA	kg/m <sup>3</sup>	-	160	320	-	160	320
	GGBS	kg/m <sup>3</sup>	-	-	-	640	480	320
	Sand	kg/m <sup>3</sup>	1280	1280	1280	1280	1280	1280
	Water	kg/m <sup>3</sup>	256	256	288	-	-	-
	AAS	kg/m <sup>3</sup>	-	-	-	256	256	288
Mix	Density	kg/m <sup>3</sup>	2203	2197	2168	2200	2178	2121
Specimen Size	Dia	mm	50	50	50	50	50	50
	Height	mm	100	100	100	100	100	100
Curing			Water	Water	Water	Air	Air	Air

**Table5. Basic Data For Ecology Computations**

Material	EE	ECO2e	Cost
	MJ/Kg	kgCO2e/kg	Rs/kg
OPC	4.798	0.80	7
GGBS	1.600	0.083	3
Fly ash	0.1004	0.0104	1
Sand	0.081	0.00516	1.25
NaOH flakes	20.5	3.2	30
Na <sub>2</sub> SiO <sub>3</sub>	10.2	2.0	12
H <sub>2</sub> O	0.25	0.001	1
Processing for Mortar with SSS (20%extracost)	0.014	0.018	0.5

**Table6:Strength properties of FC/FGP mortar**

Mix Id	Compressive Strength fc (MPa)				Tensile strength ft (MPa)	Water Absorption (%)
	7 Day	14 Day	28 Day	90 day	28 Day	90 day
CM1	17	28	30	32	3	8.9
CM2	17	28	32	33	4.1	8.5
CM3	15	21	24	26	3.9	7.9
GM1	33	37	40	40	3.9	7.2
GM2	37	40	42	45	4.1	7
GM3	27	34	39	44	3.9	6.5

The compressive strength and tensile strength data on an average of 5 specimens of each mix are given in table 6.

### 3.2. Flexural strength of Ferrocement Beams:

A beam of size 500mmX100mm and 50 mm thick is made with both conventional cement mortar and geopolymer mortar. The ingredients of mortar were mixed using an electrically operated mixer machine. The wooden mould of FC/FGC was filled fresh mortar mix with uniform distribution of reinforcement 1 layer of welded wire mesh covered with 2 and 4 layers of chicken mesh shown in Fig1. Vibration was almost not required since the mix used was self compacting in nature. The moulds were covered with wet gunny bag cloths after the bleed water of the mortar had just disappeared. The specimens were demoulded after 24 hours and then submerged in water and air for further curing. Two point loads were applied on the beam to measure the ultimate load carrying capacity of the beam Fig2.



Fig 1. Casting of ferrocement and ferro-geopolymer mortar beams

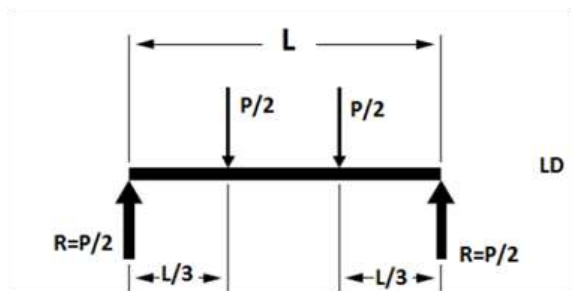


Fig 2 Two point load on beams

### 3.3. Structural load test of Ferrocement/Ferro Geopolymer panels/slabs:

The slabs of size 1000mmx300mm and 25 mm thick are made with both conventional cement mortar and geopolymer mortar with the steel reinforcement of 3mm diameter welded wire mesh with 2 and 4 layers of chicken mesh on either side of the welded wire mesh. Single point load was applied on both the slabs and measure the deflection with the corresponding loads Fig3.



Figure 3: Testing of Ferrocement slabs

### 3.4. FESEM:

Environmental Scanning Electron Microscopy, ESEM, in combination with Energy Dispersive X-ray Spectroscopy, EDS (Quanta 200 FEG SEM, operating at 30 kV in the low vacuum mode using a back scattering detector) were performed on external surfaces (depth 1 mm from the external surface) of concrete. The samples for the analysis of cross section were then impregnated under vacuum with a low viscosity epoxy resin and polished before the observation. The SEM with EDS analysis were performed for the specimens cut from the cube after curing for 28 days.

### 3.5: Accelerated Corrosion Tests:

As Portland cement matrix and Geopolymer matrix provides excellent optimum alkaline environment (pH ranging from 12.6 to 13.5) so that the steel remains protected from corrosion. Thus, there is a need for adoption of a suitable corrosion testing method for getting the test data within short duration of time.

Towards this, Alternate Wetting and Drying (AWD) Cycles were adopted; Each cycle consisted of 16 hours of soaking in 5% NaCl solution followed by oven drying of the specimens at 60°C for 8 hours. The oven dried specimens were cooled to room temperature before soaking them again in 5% NaCl solution. OPC and GPC panels are made with size of (150mm x 150mm x 25mm) with the Powdery material: sand ratio is 1:2 with 0.45 Liquid/solid ratio **Fig4 & Fig5**. The FC specimens were cured only for 3 days in water and then air dried for 3 days before subjecting them to ACT-AWD. The Ferro Geopolymer was cured only 3 days in atmospheric air before subjecting them to ACT-AWD **Fig5 & Fig6**. This short curing period of water was adopted to generate porous matrix so that faster corrosion of embedded steel is made feasible.



**Fig4: Ferro cement panels**



**Fig 5: Panels left for drying**



**Fig6: Soaking in 5% NaCl Sol**



**Fig 7: Oven dried at 60°C**



**Fig8. Electrochemical Potential Measurement using Saturated Calomel reference Electrode and Multimeter**

**3.5.1 Visual Observation Test:**

The AWD cycle is done by allowing the specimens to oven dry at 60°C for 8 hours and then soaking in 5% NaCl for 16 hours in Fig7. Before placed in NaCl solution the specimens were cooled to room temperature. Every 5 cycles the corrosion spots in the specimen is observed by visual observation.

**3.5.2 Electrochemical Potential Measurements:**

Calomel Electrode was used as reference electrode to measure the Electro chemical potential (ECP) the embedded steel and the Reference Calomel Electrode were kept on the surface of the specimens to measure the potential Fig8. More positive of ECP is expected to represent lower rates of corrosion.

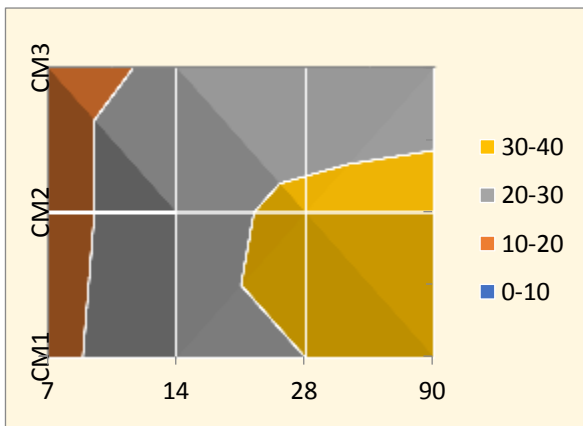
**3.6. Ecology of FC and FGC Mixes:**

Using the basic data given in Table5, the Embodied Energy (EE), Embodied CO2 Emission (ECO2e) and the cost economics contents of FC mixes and Ferro Geo mixes were computed.

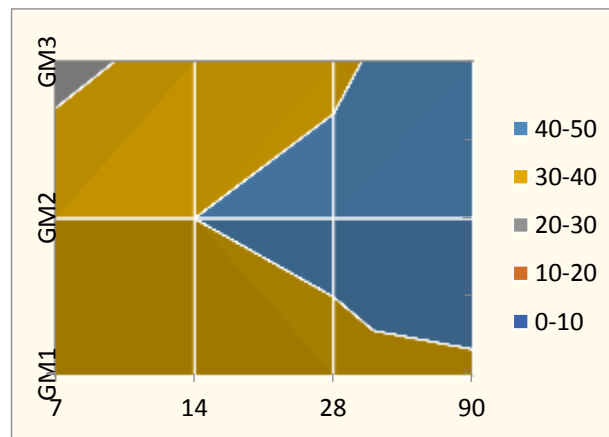
**4. Results and discussion:**

**4.1. Compressive strength:**

The compressive strength of ferropolymer matrix was increased by 25-30% of 28 days and 90 days of curing (Fig9 & Fig 10). Eventually geopolymer matrix attains maximum strength after 7 days of curing. The test results are tabulated in table6. Lesser water absorption was observed in Geopolymer matrix.



**Fig9 Conventional cement mortar (CM) Compressive strength**



**Fig 10 Geopolymer mortar (GM) Compressive strength**

**4.2 Flexural strength of Ferrocement Beams:**

The test results of two points loading there is no change in flexural behavior of FC and FGP matrices in terms of initial and ultimate carrying capacity. The flexure failure of the beam section as shown in Fig11 and the test results are tabulated in table7.

**Table7 Flexure test on Ferrocement Beam**

Mix Id	Type		CM1	CM <sub>2</sub>	GM <sub>1</sub>	GM <sub>2</sub>
<b>Two point loading test on Beam (500x100x50mm)</b>	First Crack Load	KN	3	3	3	3
			3	4	4	3
2 Layer	Ultimate Load	KN	5	5	6	6
4 Layer	Ultimate Load	KN	6	6	6	6



**Table 8. Structural load test of Ferrocement/Ferro Geopolymer panels/slabs:**

Mix Id	Type		CM1		GM <sub>1</sub>		GM <sub>2</sub>		
			E	T	E	T	E	T	
<b>Flexure Test on FC Slabs</b> (1000mmx300mmx25mm)	Deflection @ <b>P=4KN</b>	2 Layer	mm	3.8	5.67	0.69	5.04	0.68	4.79
		4 Layer	mm	4.23	5.67	0.49	5.04	0.69	4.79
	Deflection @ <b>P=</b> <b>8KN</b>	2 Layer	mm	7.65	11.65	1.2	10.09	1.28	9.58
		4 Layer	mm	8.58	11.65	1.13	10.09	1.37	9.58
	Deflection @ <b>P=</b> <b>12KN</b>	2 Layer	mm	10.7	17.03	1.86	15.12	2.04	14.37
		4 Layer	mm	12.7	17.03	1.74	15.12	1.95	14.37

E – Experimental Value

T – Theoretical Value

**Figure11:Initial crack and Failure of beam****4.3.Structural load test of Ferrocement/Ferro Geopolymer panels/slabs:**

The observed test results of the Ferrocement and ferrogeopolymer slabs are tabulated in **table8**. The result indicates that the readings obtained from the experiments showing lesser deformation than the actual calculated theoretical deformations. The difference between actual theoretical and experimental deformation was about 60-70% higher in ferrocement panels and 500-700% in ferrogeopolymer panels. As a result ferrogeopolymer panels are high stiffness material than the conventional ferrocement panels.

Theoretical Deformation of the ferrocement / ferrogeopolymer slabs were calculated by using the formula

$$\delta = \frac{PL^3}{48EI}$$

Where,

$\delta$  = Deflection of the slab corresponding to the applied load

P = Applied Live load to the Slab

L = Effective Length of the slab

E = young's Modulus of the slab

I = Moment of Inertia of the member

Here  $E = E_s + E_c$

$I = I_s + I_c$

**4.4FESEM:**

Calcium silicate hydrate plates and calcium hydroxide crystals were clearly identifiable in the CM1 specimen. Well compacted rods and grains are typical hydration products of mature cement paste. (Fig 12.0) Dense micro structure does not allow the diffusion of chloride content. From **Fig.13** it is observed that most of the slag particles are dissolved by the reaction generating solution. The dense micro structure with micro cracks was observed it is due to the formation of C-S-H gel. From the **fig. 14** it is observed that there is unreacted fly ash particle with spherical shape. The microstructure was improved after 28 days it is due to the slow



dissolution of fly ash particle in the reaction generating solution and it is reflected in the mechanical strength property.

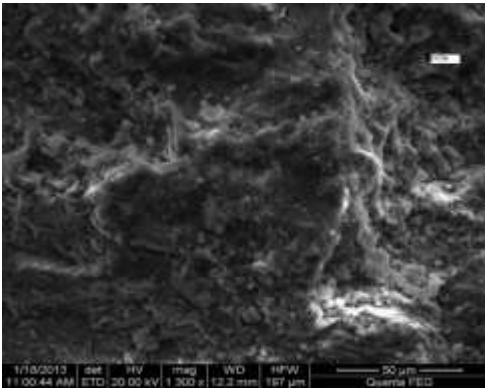


Fig 12:SEM and EDS of CM1 matrix

Element	At.Wt %
C	23.5
O	36.2
Al	7.8
Si	10.7
Fe	0.74
Ca	21.4

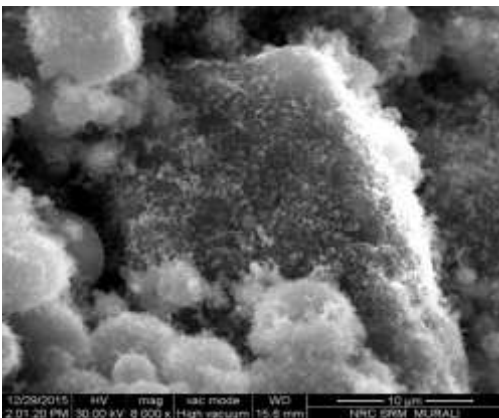


Fig 13. SEM and EDX of GM1

Element	At.Wt(%)
Na	2.7
Ca	33.4
Al	4.5
Si	8.7
O	32.2
C	18.5

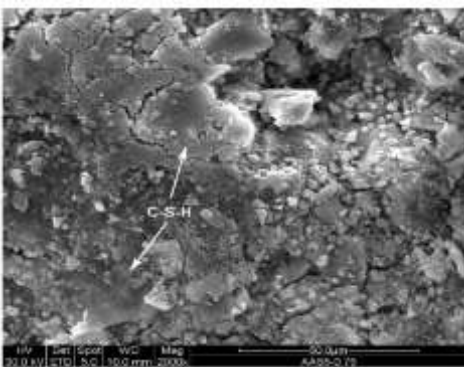


Fig 14. SEM and EDAX of GM2 matrix

Element	At.Wt%
C	44.19
O	44.91
Al	00.79
Si	02.93
Ca	06.90
Fe	00.29



Figure15:Corrosion spots observed in specimens

#### 4.5 Corrosion test:

##### 4.5.1 Visual Observation Test:

Visual observation of corrosion spots was done on every 5 cycles of AWD. After 90 and 180 cycles of AWD the specimens made with ferrocement matrix shows more corrosion spots than the ferropolymer matrix **fig15** and the results are tabulated in **table 9**. Generally sodium silicate provides more protection to the steel reinforcement for the initiation of the corrosion.

**Table 9. Test Data after Accelerated Corrosion Testing**

Mix Id	Visual Corrosion Spots ACT-AWD Each Cycle of ACT-AWD = Soaking 5% NaCl for 16hrs + Drying at 60°C for 8hrs			ECP Value (mv)
	60 Cycles	90 Cycles	180 Cycles	90 Cycles
	CM1	2	4	6
CM2	0	2	4	-186
CM3	0	1	2	-170
GM1	0	2	3	-160
GM2	0	1	2	-154
GM3	0	0	1	-145

##### 4.5.2 Electro Chemical Potential Measurement

The potential shift is a function of the durability of the protective layer on steel surface. Thus, the corrosion potential of rebars in concrete is a good indication of either the depassivation or activation of corrosion. The corrosion potential measurements against SCE is given in Table 9.0 The ECP of -160mV recorded in FGP matrix and this value is less negative as compared to -190mV of mix FC. According to the guidelines involved in ASTM C876-87 the probability of corrosion initiation is greater than 90% when corrosion potentials are more negative than (-350 mV) relative to the copper-copper sulfate (CSE) and -270 mV relative to the Saturated Calomel Electrode. There was a 15-20% reduction in potential observed in ferropolymer matrix.

#### 4.6. Ecology of FC and FGC Mixes

Using the basic data given in **Table 5**, the Embodied Energy (EE) and the Embodied CO<sub>2</sub> Emission (ECO<sub>2e</sub>) contents of FC mixes and FGP were computed and are given in **Table 10**; the cost analysis is also worked out. The calculated values indicate that EE of FC mix is 3270MJ/m<sup>3</sup> and this value reduces to 2250MJ/m<sup>3</sup> in case of FGP mix and thus FGP requires less energy to produce. The ECO<sub>2e</sub> contents of 559 and 284kg CO<sub>2e</sub> for mixes FC and FGP indicate that FGP accounts for lower quantity of emission of Green House Gas. Thus, FGP is highly eco-friendly material. Eventually 90 day strength of FGP is higher as 40MPa than the 30 MPa of FC matrices. The actual production cost of per metre<sup>3</sup> of FGP matrix is 17% lesser than the production of FC matrices.

**Table 10. Ecology of FC and FGC Mixes**

MIXID	CM1	GM1	%change	Remarks
Cement kg/m <sup>3</sup>	640	-		
GGBFS kg/m <sup>3</sup>	-	640		
Sand kg/m <sup>3</sup>	1280	1280		
NaOH flakes kg/m <sup>3</sup>		25.6		
Na <sub>2</sub> SiO <sub>3</sub> kg/m <sup>3</sup>		51.2		

DW kg/m <sup>3</sup>	256	179.2		
Density of the matrix	2203	2200	-	Almost similar self weight
90daystr, f <sub>c</sub> 90d MPa	30	40	25	Higher strength of matrix
EE=Embodied EnergyMJ/m <sup>3</sup>	3270	2250	32	Lower Embodied Energy
ECO <sub>2</sub> e kgCO <sub>2</sub> e/m <sup>3</sup>	559	284	50	Lower CO <sub>2</sub> Emission content
CostRs/m <sup>3</sup>	7438	6181	17	Lower cost

## 5. Conclusion:

The ferropolymer shows 25-30% increase in the mechanical strength property compared to that of OPC. The ferropolymer shows 85% increase in the deflection at a higher load (1600Kg). Inclusion of fly ash shows does not affect the flexural strength compared with that of ferrocement. It is observed that there is an increase in the corrosion spots in the 90th compared with that of ferropolymer with the untreated mesh in ACT-AWD. The positive value of EP indicates that ferropolymer has low corrosion rate compared to that of ferrocement. The overall results reveals that ferrocement slab and panels shows good performance in engineering aspects and well suited for the construction field in the developed and developing countries.

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